

# **CYS National Annex to CYS EN 1997-1:2004**

## **Eurocode 7: Geotechnical Design**

### **Part 1: General rules**

Prepared by  
Eurocodes Committee, Scientific and Technical  
Chamber of Cyprus under a Ministry of Interior's Programme



**NATIONAL ANNEX**  
**TO**  
**CYS EN 1997-1:2004 Eurocode 7: Geotechnical Design**  
**Part1: General rules**

**This National Annex has been approved by the Board of Governors of the Cyprus Organisation for Standardisation on 11/06/2010.**

## INTRODUCTION

This National Annex has been prepared by the Eurocodes Committee of the Technical Chamber of Cyprus which was commissioned by the Ministry of Interior of the Republic of Cyprus

## NA 1 SCOPE

This National Annex is to be used together with CYS EN 1997-1:2004

This National Annex gives:

- (a) Nationally determined parameters for the following clauses of CYS EN 1997-1:2004 where National choice is allowed (see Section NA 2):

2.1(8)P, 2.4.6.1(4)P, 2.4.6.2(2)P, 2.4.7.1(2)P, 2.4.7.1(3), 2.4.7.2(2)P, 2.4.7.3.2(3)P, 2.7.4.3.3(2)P, 2.4.7.3.4.1(1)P, 2.4.7.4(3)P, 2.4.7.5(2)P, 2.4.8(2), 2.4.9(1)P, 2.5(1), 7.6.2.2(8)P, 7.6.2.2(14)P, 7.6.2.3(4)P, 7.6.2.3(5)P, 7.6.2.3(8), 7.6.2.4(4)P, 7.6.3.2(2)P, 7.6.3.2(5)P, 7.6.3.3(3)P, 7.6.3.3(4)P, 7.6.3.3(6), 8.5.2(2)P, 8.5.2(3), 8.6(4), 11.5.1(1)P

and for the following clauses in Annex A:

A.2, A.3.1, A.3.2, A.3.3.1, A.3.3.2, A.3.3.3, A.3.3.4, A.3.3.5, A.3.3.6, A.4, A.5.

- (b) The procedure to be used where alternative procedures are given in CYS EN 1997-1:2004 (see Section NA 3).
- (c) Decisions on the use of the Informative Annexes B, C, D, E, F, G, H and J (see Section NA 4)
- (d) References to non-contradictory complementary information to assist the user to apply CYS EN 1997-1:2004 (see Section NA 5).

## NA 2 NATIONALLY DETERMINED PARAMETERS

### NA 2.1 Clause 2.1(8)P Minimum requirements for geotechnical investigations, calculations and construction control checks

The minimum requirement for geotechnical investigations for light and simple structures and small earthworks is to obtain an indication of the stratification and groundwater levels within the zone of ground governing the behaviour of the structure or earthworks at the limit states being considered.

### NA 2.2 Clause 2.4.6.1(4)P Design values of actions

The partial factors on actions providing an appropriate level of safety for conventional designs are specified in the National Annex to CYS EN 1990:2002. They are also shown in NA 2.30, NA 2.32, NA 2.40 and NA 2.42 of this National Annex.

### NA 2.3 Clause 2.4.6.2(2)P Design values of geotechnical parameters

The partial factors for soil parameters providing a minimum level of safety for conventional designs are specified in NA 2.31, NA 2.33 and NA 2.41.

**NA 2.4 Clause 2.4.7.1(2)P Ultimate limit states – persistent and transient situations**

The partial factors in persistent and transient situations are specified in clauses NA 2.30, NA 2.31, NA 2.32, NA 2.33, NA 2.34, NA 2.35, NA 2.37, NA 2.38, NA 2.39, NA 2.40, NA 2.41 and NA 2.42.

**NA 2.5 Clause 2.4.7.1(3) Ultimate limit states – accidental situations**

All values of partial factors for actions or the effects of actions in accidental situations should be taken equal to 1,0. All values of partial factors for resistances should then be selected according to the particular circumstances of the accidental situation.

For seismic design situations, all values of partial factors for actions, the effects of actions and resistances should be taken equal to 1,0. The partial factors for ground strength parameters are specified in the National Annex to CYS EN 1998-5:2003. The full requirements for seismic design are given in CYS EN 1998-5:2003.

**NA 2.6 Clause 2.4.7.2(2)P Verification of static equilibrium (EQU)**

The partial factors for persistent and transient situations are specified in NA 2.30 and NA 2.31.

**NA 2.7 Clause 2.4.7.3.2(3)P Verification of STR and GEO limit states – design effects of actions**

The partial factors in persistent and transient situations are specified in NA 2.32 and NA 2.33.

**NA 2.8 Clause 2.4.7.3.3(2)P Verification of STR and GEO limit states – design resistances**

The partial factors in persistent and transient situations are specified in NA 2.34, NA 2.35, NA 2.37, NA 2.38 and NA 2.39.

**NA 2.9 Clause 2.4.7.3.4.1(1)P Verification of STR and GEO limit states – design approach**

For the verification of the structural (STR) and geotechnical (GEO) limit states under persistent and transient situations Design Approach 2 shall be used.

**NA 2.10 Clause 2.4.7.4(3)P Verification of uplift (UPL)**

The partial factors for persistent and transient situations are specified in NA 2.40 and NA 2.41.

**NA 2.11 Clause 2.4.7.5(2)P Verification of resistance to failure by heave (HYD)**

The partial factors for persistent and transient situations are specified in the National Annex to CYS EN 1990:2002. They are also shown in NA 2.42 of this National Annex.

**NA 2.12 Clause 2.4.8(2) Serviceability limit states**

Values of partial factors for serviceability limit states should be taken as equal to 1.0.

**NA 2.13 Clause 2.4.9(1)P Limiting values for movements of foundations**

Permitted foundation movements shall be established for each structure under consideration. In the absence of specified limiting values of structural deformations of the supported structure, the guidance given on structural deformation and foundation movement in Annex H may be used.

**NA 2.14 Clause 2.5(1) Design by prescriptive measures**

The prescriptive measures should be based on comparable experience as defined in clause 1.5.2.2 of CYS EN 1997-1:2004.

**NA 2.15 Clause 7.6.2.2(8)P Ultimate compressive pile resistance from static load tests – correlation factors**

The values of the correlation factors are specified in NA 2.36.

**NA 2.16 Clause 7.6.2.2(14)P Ultimate compressive pile resistance from static load tests – partial resistance factors**

The partial factors in persistent and transient situations are specified in NA 2.35.

**NA 2.17 Clause 7.6.2.3(4)P Ultimate compressive pile resistance from ground test results – partial resistance factors**

The partial factors in persistent and transient situations are specified in NA 2.35.

**NA 2.18 Clause 7.6.2.3(5)P Ultimate compressive pile resistance from ground test results – correlation factors**

The values of the correlation factors are specified in NA 2.36.

**NA 2.19 Clause 7.6.2.3(8) Ultimate compressive pile resistance from ground test results – alternative method**

Model factors of at least 1,25 (depending on the number and variability of ground test result profiles used in the determination of base resistance and shaft resistance) should be applied to the partial factors  $\gamma_b$  and  $\gamma_s$  when using the alternative method described in Clause 7.6.2.3(8) of CYS EN 1997-1:2004.

**NA 2.20 Clause 7.6.2.4(4)P Ultimate compressive pile resistance from dynamic impact tests – correlation factors**

The values of the correlation factors are specified in NA 2.36.

**NA 2.21 Clause 7.6.3.2(2)P Ultimate tensile pile resistance from pile load tests – partial resistance factor**

The partial factors in persistent and transient situations are specified in NA 2.35.

**NA 2.22 Clause 7.6.3.2(5)P Ultimate tensile pile resistance from pile load tests – correlation factors**

The values of the correlation factors are specified in NA 2.36.

**NA 2.23 Clause 7.6.3.3(3)P Ultimate tensile pile resistance from ground test results – partial resistance factor**

The partial factors in persistent and transient situations are specified in NA 2.35.

**NA 2.24 Clause 7.6.3.3(4)P Ultimate tensile pile resistance from ground test results – correlation factors**

The values of the correlation factors are specified in NA 2.36.

**NA 2.25 Clause 7.6.3.3(6) Ultimate tensile pile resistance from ground test results – alternative method**

A model factor of at least 1,25 (depending on the number and variability of ground test result profiles used in the determination of shaft resistance) should be applied to the partial factor  $\gamma_{s,t}$  when using the alternative method described in Clause 7.6.3.3(6) of CYS EN 1997-1:2004.

**NA 2.26 Clause 8.5.2(2)P Anchorage pull-out resistance determined from the results of suitability tests – partial resistance factor**

The partial factors in persistent and transient situations are specified in NA 2.37.

**NA 2.27 Clause 8.5.2(3) Anchorage pull-out resistance determined from the results of suitability tests – correlation factor**

The correlation factor  $\xi_a$  must be based on comparable experience as defined in clause 1.5.2.2 of CYS EN 1997-1:2004. In the absence of comparable experience, the characteristic value  $R_{a,k}$  should be related to the test results through the correlation factors  $\xi_{a,1}$  and  $\xi_{a,2}$  that take account of the number of tests and the variability of the test results in the following way:

$$R_{a,k} = \text{Min} \left\{ \frac{(R_{a,m})_{\text{mean}}}{\xi_{a,1}}; \frac{(R_{a,m})_{\text{min}}}{\xi_{a,2}} \right\}$$

where  $R_{a,m}$  is the measured value of  $R_a$  in one or several anchorage suitability tests and  $\xi_{a,1}$  and  $\xi_{a,2}$  are the correlation factors specified in Table 8.1(CYS) below.

**Table 8.1(CYS): Correlation factors  $\xi_a$  to derive characteristic values from anchorage suitability tests ( $n$  - number of suitability tests)**

$\xi_a$ for $n$	1	2	$\geq 3$
=			
$\xi_{a,1}$	1,20	1,15	1,10
$\xi_{a,2}$	1,20	1,13	1,05

**NA 2.28 Clause 8.6(4) Serviceability limit state anchorage design**

A model factor of 1,35 should be applied to the calculated SLS anchorage force. The ULS design of the anchorages should then be performed with a design value of the anchor load equal to the greater of  $P_d$  and  $1,35P_k$  where  $P_d$  results from the ULS design of the structure and  $P_k$  results from the SLS design of the structure.

**NA 2.29 Clause 11.5.1(1)P Stability analysis for slopes – partial factors**

The partial factors for persistent and transient situations are specified in NA 2.32, NA 2.33 and NA 2.39.

### NA 2.30 Clause A.2(1)P Partial factors for equilibrium state (EQU) verification – on actions

For the verification of the equilibrium state (EQU), the values of the partial factors  $\gamma_{G;dst}$ ,  $\gamma_{G;stb}$ ,  $\gamma_{Q;dst}$  and  $\gamma_{Q;stb}$  are specified in the National Annex to CYS EN 1990:2002, Table A1.2(A) (in which the subscripts *sup* and *inf* correspond with *dst* and *stb*). These values are also shown in Table A.1(CYS) below.

**Table A.1(CYS): Partial factors on actions ( $\gamma_F$ )**

Action	Symbol	Value
Permanent Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{G;dst}$	1,1
	$\gamma_{G;stb}$	0,9
Variable Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{Q;dst}$	1,5
	$\gamma_{Q;stb}$	0
<sup>a</sup> Destabilising <sup>b</sup> Stabilising		

### NA 2.31 Clause A.2(2)P Partial factors for equilibrium state (EQU) verification – for soil parameters

For the verification of the equilibrium state (EQU), the values of the partial factors  $\gamma_{\phi'}$ ,  $\gamma_{c'}$ ,  $\gamma_{cu}$ ,  $\gamma_{qu}$  and  $\gamma_{\gamma}$  are specified in Table A.2(CYS).

**Table A.2(CYS): Partial factors for soil parameters ( $\gamma_M$ )**

Soil parameter	Symbol	Value
Angle of shearing resistance <sup>a</sup>	$\gamma_{\phi'}$	1,25
Effective cohesion	$\gamma_{c'}$	1,25
Undrained shear strength	$\gamma_{cu}$	1,4
Unconfined strength	$\gamma_{qu}$	1,4
Weight density	$\gamma_{\gamma}$	1,0
<sup>a</sup> This factor is applied to $\tan \phi'$		

### NA 2.32 Clause A.3.1(1)P Partial factors for structural (STR) and geotechnical (GEO) limit states verification – on actions or the effects of actions

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_G$  and  $\gamma_Q$  are specified in the National Annex to CYS EN 1990:2002,

Tables A1.2(B) and A1.2(C). These values are also shown in Table A.3(CYS) below for the set *AI*.

**Table A.3(CYS): Partial factors on actions ( $\gamma_F$ ) or the effects of actions ( $\gamma_E$ )**

Action		Symbol	Set
			<i>AI</i>
Permanent	Unfavourable	$\gamma_G$	1,35
	Favourable		1,0
Variable	Unfavourable	$\gamma_Q$	1,5
	Favourable		0

**NA 2.33 Clause A.3.2(1)P Partial factors for structural (STR) and geotechnical (GEO) limit states verification – for soil parameters**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_{\phi'}$ ,  $\gamma_{c'}$ ,  $\gamma_{cu}$ ,  $\gamma_{qu}$  and  $\gamma_{\gamma}$  are specified in Table A.4(CYS) for the set *MI*.

**Table A.4(CYS): Partial factors for soil parameters ( $\gamma_M$ )**

Soil parameter	Symbol	Set
		<i>MI</i>
Angle of shearing resistance <sup>a</sup>	$\gamma_{\phi'}$	1,0
Effective cohesion	$\gamma_{c'}$	1,0
Undrained shear strength	$\gamma_{cu}$	1,0
Unconfined strength	$\gamma_{qu}$	1,0
Weight density	$\gamma_{\gamma}$	1,0
<sup>a</sup> This factor is applied to $\tan \phi'$		

**NA 2.34 Clause A.3.3.1(1)P Partial resistance factors for spread foundations**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_{R;v}$  and  $\gamma_{R;h}$  are specified in Table A.5(CYS) for the set *R2*.

**Table A.5(CYS): Partial resistance factors ( $\gamma_R$ ) for spread foundations**

Resistance	Symbol	Set
		<i>R2</i>
Bearing	$\gamma_{R,v}$	1,4
Sliding	$\gamma_{R,h}$	1,1

**NA 2.35 Clause A.3.3.2(1)P Partial resistance factors for pile foundations**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_b$ ,  $\gamma_s$ ,  $\gamma_t$  and  $\gamma_{s,t}$  are specified in Table A.6(CYS) for driven piles, in Table A.7(CYS) for bored piles and in Table A.8(CYS) for continuous flight auger (CFA) piles, all for the set *R2*.

**Table A.6(CYS): Partial resistance factors ( $\gamma_R$ ) for driven piles**

Resistance	Symbol	Set
		<i>R2</i>
Base	$\gamma_b$	1,1
Shaft (compression)	$\gamma_s$	1,1
Total/combined (compression)	$\gamma_t$	1,1
Shaft in tension	$\gamma_{s,t}$	1,15

**Table A.7(CYS): Partial resistance factors ( $\gamma_R$ ) for bored piles**

Resistance	Symbol	Set
		<i>R2</i>
Base	$\gamma_b$	1,1
Shaft (compression)	$\gamma_s$	1,1
Total/combined (compression)	$\gamma_t$	1,1
Shaft in tension	$\gamma_{s,t}$	1,15

**Table A.8(CYS): Partial resistance factors ( $\gamma_R$ ) for continuous flight auger (CFA) piles**

Resistance	Symbol	Set
		$R2$
Base	$\gamma_b$	1,1
Shaft (compression)	$\gamma_s$	1,1
Total/combined (compression)	$\gamma_t$	1,1
Shaft in tension	$\gamma_{s;t}$	1,15

**NA 2.36 Clause A.3.3.3(1)P Correlation factors for pile foundations**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the correlation factors  $\xi_1$ ,  $\xi_2$ ,  $\xi_3$ ,  $\xi_4$ ,  $\xi_5$  and  $\xi_6$  are specified in Table A.9(CYS), Table A.10(CYS) and Table A.11(CYS).

**Table A.9(CYS): Correlation factors  $\xi$  to derive characteristic values from static pile load tests ( $n$  - number of tested piles)**

$\xi$ for $n =$	1	2	3	4	$\geq 5$
$\xi_1$	1,40	1,30	1,20	1,10	1,00
$\xi_2$	1,40	1,20	1,05	1,00	1,00

**Table A.10(CYS): Correlation factors  $\xi$  to derive characteristic values from ground test results ( $n$  - the number of profiles of tests)**

$\xi$ for $n =$	1	2	3	4	5	7	10
$\xi_3$	1,40	1,35	1,33	1,31	1,29	1,27	1,25
$\xi_4$	1,40	1,27	1,23	1,20	1,15	1,12	1,08

**Table A.11(CYS): Correlation factors  $\xi$  to derive characteristic values from dynamic impact tests<sup>a, b, c, d, e</sup> ( $n$  - number of tested piles)**

$\xi$ for $n =$	$\geq 2$	$\geq 5$	$\geq 10$	$\geq 15$	$\geq 20$
$\xi_5$	1,60	1,50	1,45	1,42	1,40
$\xi_6$	1,50	1,35	1,30	1,25	1,25

<sup>a</sup> The  $\xi$ -values in the table are valid for dynamic impact tests.

<sup>b</sup> The  $\xi$ -values may be multiplied with a model factor of 0,85 when using dynamic impact tests with signal matching.

<sup>c</sup> The  $\xi$ - values should be multiplied with a model factor of 1,10 when using a pile driving formula with measurement of the quasi-elastic pile head displacement during the impact.

<sup>d</sup> The  $\xi$ -values shall be multiplied with a model factor of 1,20 when using a pile driving formula without measurement of the quasi-elastic pile head displacement during the impact.

<sup>e</sup> If different piles exist in the foundation, groups of similar piles should be considered separately when selecting the number  $n$  of test piles.

**NA 2.37 Clause A.3.3.4(1)P Partial resistance factors for pre-stressed anchorages**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_{a,t}$  and  $\gamma_{a,p}$  are specified in Table A.12(CYS) for the set R2.

**Table A.12(CYS): Partial resistance factors ( $\gamma_R$ ) for pre-stressed anchorages**

Resistance	Symbol	Set
		R2
Temporary	$\gamma_{a,t}$	1,1
Permanent	$\gamma_{a,p}$	1,1

**NA 2.38 Clause A.3.3.5(1)P Partial resistance factors for retaining structures**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the values of the partial factors  $\gamma_{R,v}$ ,  $\gamma_{R,h}$  and  $\gamma_{R,c}$  are specified in Table A.13(CYS) for the set R2.

**Table A.13(CYS): Partial resistance factors ( $\gamma_R$ ) for retaining structures**

Resistance	Symbol	Set
		<i>R2</i>
Bearing capacity	$\gamma_{R:v}$	1,4
Sliding resistance	$\gamma_{R:h}$	1,1
Earth resistance	$\gamma_{R:e}$	1,4

**NA 2.39 Clause A.3.3.6(1) Partial resistance factors for slopes and overall stability**

For the verification of the structural (STR) and geotechnical (GEO) limit states, the value of the partial factor  $\gamma_{R:e}$  is specified in Table A.14(CYS) for the set *R2*.

**Table A.14(CYS): Partial resistance factor ( $\gamma_R$ ) for slopes and overall stability**

Resistance	Symbol	Set
		<i>R2</i>
Earth resistance	$\gamma_{R:e}$	1,1

**NA 2.40 Clause A.4(1) Partial factors for uplift limit state (UPL) verifications – on actions**

For the verification of the uplift limit state (UPL), the values of the partial factors  $\gamma_{G:dst}$ ,  $\gamma_{G:stb}$ ,  $\gamma_{Q:dst}$  and  $\gamma_{Q:stb}$  are specified in Table A.15(CYS).

**Table A.15(CYS): Partial factors on actions ( $\gamma_F$ )**

Action	Symbol	Value
Permanent Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{G:dst}$	1,0
	$\gamma_{G:stb}$	0,9
Variable Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{Q:dst}$	1,5
	$\gamma_{Q:stb}$	0
<sup>a</sup> Destabilising;		
<sup>b</sup> Stabilising		

**NA 2.41 Clause A.4(2)P Partial factors for uplift limit state (UPL) verifications – for soil parameters and resistances**

For the verification of the uplift limit state (UPL), the values of the partial factors  $\gamma_{\phi'}$ ,  $\gamma_{c'}$ ,  $\gamma_{cu}$ ,  $\gamma_{s;t}$  and  $\gamma_a$  are specified in Table A.16(CYS).

**Table A.16(CYS): Partial factors for soil parameters and resistances**

Soil parameter	Symbol	Value
Angle of shearing resistance <sup>a</sup>	$\gamma_{\phi'}$	1,25
Effective cohesion	$\gamma_{c'}$	1,25
Undrained shear strength	$\gamma_{cu}$	1,40
Tensile pile resistance	$\gamma_{s;t}$	1,40
Anchorage resistance	$\gamma_a$	1,40
<sup>a</sup> This factor is applied to $\tan \phi'$		

**NA 2.42 Clause A.5(1)P Partial factors for hydraulic heave limit state (HYD) verification**

For the verification of the hydraulic heave limit state (HYD), the values of the partial factors  $\gamma_{G;dst}$ ,  $\gamma_{G;stb}$ ,  $\gamma_{Q;dst}$  and  $\gamma_{Q;stb}$  are specified in Table A.17(CYS).

**Table A.17(CYS): Partial factors on actions ( $\gamma_F$ )**

Action	Symbol	Value
Permanent Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{G;dst}$	1,35
	$\gamma_{G;stb}$	0,90
Variable Unfavourable <sup>a</sup> Favourable <sup>b</sup>	$\gamma_{Q;dst}$	1,50
	$\gamma_{Q;stb}$	0
<sup>a</sup> Destabilising <sup>b</sup> Stabilising		

## **NA 3 THE PROCEDURE TO BE USED WHEN ALTERNATIVE PROCEDURES ARE GIVEN IN CYS EN 1997-1:2004**

### **NA 3.1 Clause 2.4.7.3.4.1(1)P Choice of design approach for the verification of Structural (STR) and Geotechnical (GEO) limit states**

As indicated in NA 2.9, Design Approach 2 shall be used.

## **NA 4 DECISION ON USE OF THE INFORMATIVE ANNEXES B, C, D, E, F, G, H AND J**

### **NA 4.1 Annex B**

Annex B may be used

### **NA 4.2 Annex C**

Annex C may be used

### **NA 4.3 Annex D**

Annex D may be used

### **NA 4.4 Annex E**

Annex E may be used

### **NA 4.5 Annex F**

Annex F may be used

### **NA 4.6 Annex G**

Annex G may be used

### **NA 4.7 Annex H**

Annex H may be used

### **NA 4.8 Annex J**

Annex J may be used

## **NA 5 REFERENCES TO NON-CONTRADICTIONARY COMPLEMENTARY INFORMATION**

None



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